

**DESIGN PROPOSAL WITH FIBROGRID AS
REINFORCEMENT GEOGRIDS WITH KEYSTONE WALL AS
FRONT FAÇADE FOR THE REHABILITATION WORKS
NEAR THE DEWAN AT JALAN GENTAM SEBERANG,
DAERAH KUALA PILAH, NEGERI SEMBILAN.**

Dec 8. 2023

Ref: DC 23012-N-R1

Prepared by:



MTS FIBROMAT (M) SDN. BHD.

En. Mazlan

Tel: 019 3616528

email: Mazlan.mts@gmail.com

DESIGN PROPOSAL WITH FIBROGRID AS REINFORCEMENT GEOGRIDS WITH
KEYSTONE WALL AS FRONT FAÇADE FOR THE REHABILITATION WORKS
NEAR THE DEWAN AT JALAN KG. GENTAM SEBERANG, SAWAH LEBAR,
DAERAH KUALA PILAH, NEGERI SEMBILAN.

– *Technical Proposal*

Table of Content

1.0 INTRODUCTION 1

2.0 ASSUMPTIONS..... 2

3.0 DESIGN & ANALYSIS 2

4.0 RECOMMENDATIONS & CONCLUSIONS 3

5.0 LIMITATIONS..... 4

Appendix A – Slope Stability Analysis Results

Appendix B – Drawings

Appendix C – Specifications

DESIGN PROPOSAL WITH FIBROGRID AS REINFORCEMENT GEOGRIDS WITH KEYSTONE WALL AS FRONT FAÇADE FOR THE REHABILITATION WORKS NEAR THE DEWAN AT JALAN KG. GENTAM SEBERANG, SAWAH LEBAR, DAERAH KUALA PILAH, NEGERI SEMBILAN.

– Design Proposal

1.0 INTRODUCTION

This report presents the design proposal using Keystone Wall stabilization works system for the rehabilitation works near the Dewan at Jalan Kg. Gentam Seberang, Sawah Lebar, Daerah Kuala Pilah, Negeri Sembilan Darul Khusus.

The design and analysis methodology and assumptions will be first presented, followed by the evaluation of analysis results and finally recommendations for the proposed slope stabilization works to be installed will be highlighted.

Based on the information gather, it showed that the height of the wall varies between 1.5m to mm high for a length of approximately 250m. We will base on the average height of 2.5m for the estimation cost purposes.

Pictures





2.0 ASSUMPTIONS

Due to the lack of information about the site (history, hydrology, subsoil & topography survey data, underground services, etc.), some assumptions were made to accomplish the design proposal based on the knowledge and engineering judgements of the designer. All engineering assumptions have to be verified and confirmed by soil investigation and topography survey. It is also difficult to assume the quantity as we do not know what the existing angle of slope is and also the amount of earth that needs to be excavated and refill with good material.

3.0 DESIGN & ANALYSIS

Section of the proposed slope rehabilitation was modelled and checked for stability. The acceptable slope stability safety factor for the proposed slope treatment is set to minimum 1.5.

Using the reinforced slope stability analysis German software GGU Stability, numerous trial slip circles were tried out and the slip for the lowest safety factor was presented.

The related design parameters are shown in the next page

DESIGN PROPOSAL WITH FIBROGRID AS REINFORCEMENT GEOGRIDS WITH KEYSTONE WALL AS FRONT FAÇADE FOR THE REHABILITATION WORKS NEAR THE DEWAN AT JALAN KG. GENTAM SEBERANG, SAWAH LEBAR, DAERAH KUALA PILAH, NEGERI SEMBILAN.

– Technical Proposal

a. Soil Engineering Parameters

Layer (top down)	Description	Soil Model	Bulk unit weight, γ (kN/m ³)	Design cohesion, C' (kN/m ²)	Design soil internal friction, ϕ' (°)
1	Backfill soil or Reinforced soil	Mohr-Coulomb	18	5	28
2	Existing/Retained soil	Mohr-Coulomb	18	3	30
3	Foundation Soil (original)	Mohr-Coulomb	18	5	30
4	Foundation Aggregate (compacted)	Mohr-Coulomb	20	0	34

b. FibroGrid Parameters

Ultimate Tensile, T_u (kN/m)	Installation damage factor, f_1	Durability factor, f_2	Creep factor, f_3	Design Value, T_{des} (kN/m)
FG 60	1.05	1.1	1.47	36

The allowable long-term design tensile for the reinforcement FG60 for example,

$$T_a = T_u / (f_1 + f_2 + f_3) = 36 \text{ kN/m}$$

Both the internal and global stabilities were checked and the lowest safety factors were determined.

The result of the analysis shows satisfactory safety factor using an approximate height of the Keystone Wall of 1.5m high in a single tier up to 3.5m high with Reinforcing FibroGrid elements, FG 60/30 as shown in the drawings in Appendix B.

The factors of Safety (FOS) were shown in the analyses attached in Appendix A. The table below summarizes the different FOS for different wall heights.

4.0 RECOMMENDATIONS & CONCLUSIONS

Basically, our proposal with Keystone Wall design is a combination of the following elements:

1. Drainage controls – subsoil drainage media to cater for the under-ground water and infiltration during and after construction.
2. Wall reinforcement – the use of FibroGrid geogrid as reinforcement behind the wall system not only stabilized the slope but also reduced the cost as compared to RC wall system.

– Technical Proposal

3. Temporary protection – the excavation for the wall construction shall ensure the short-term stability of the excavation slopes.

The proposed design can be constructed easily without the need of heavy machinery. Excellent flexibility aspect of the geogrid-FibroGrid as reinforcement and with Keystone wall system as the fascia is another advantage over the conventional rigid wall system like RC wall. We believe the above proposal suits the tight budget constraint of the project without sacrificing the stability aspect.

5.0 LIMITATIONS

The design proposal was performed using the degree of care and skill ordinarily exercised, under similar circumstances, by engineers practising in this or similar localities. No warranty, expressed or implied, is made as to the conclusions and professional advice included in this report.

The assumptions and engineering judgements made to accomplish the proposal are believed to be representative of the entire contract area. However, as revealed by many previous developments, the actual soil and geological conditions faced during the construction may vary drastically from the soil investigation result. If this occurs, the changed of conditions must be evaluated by geotechnical engineer and revision or alteration of design may be made accordingly.

Appendix A

Slope Stability Analysis

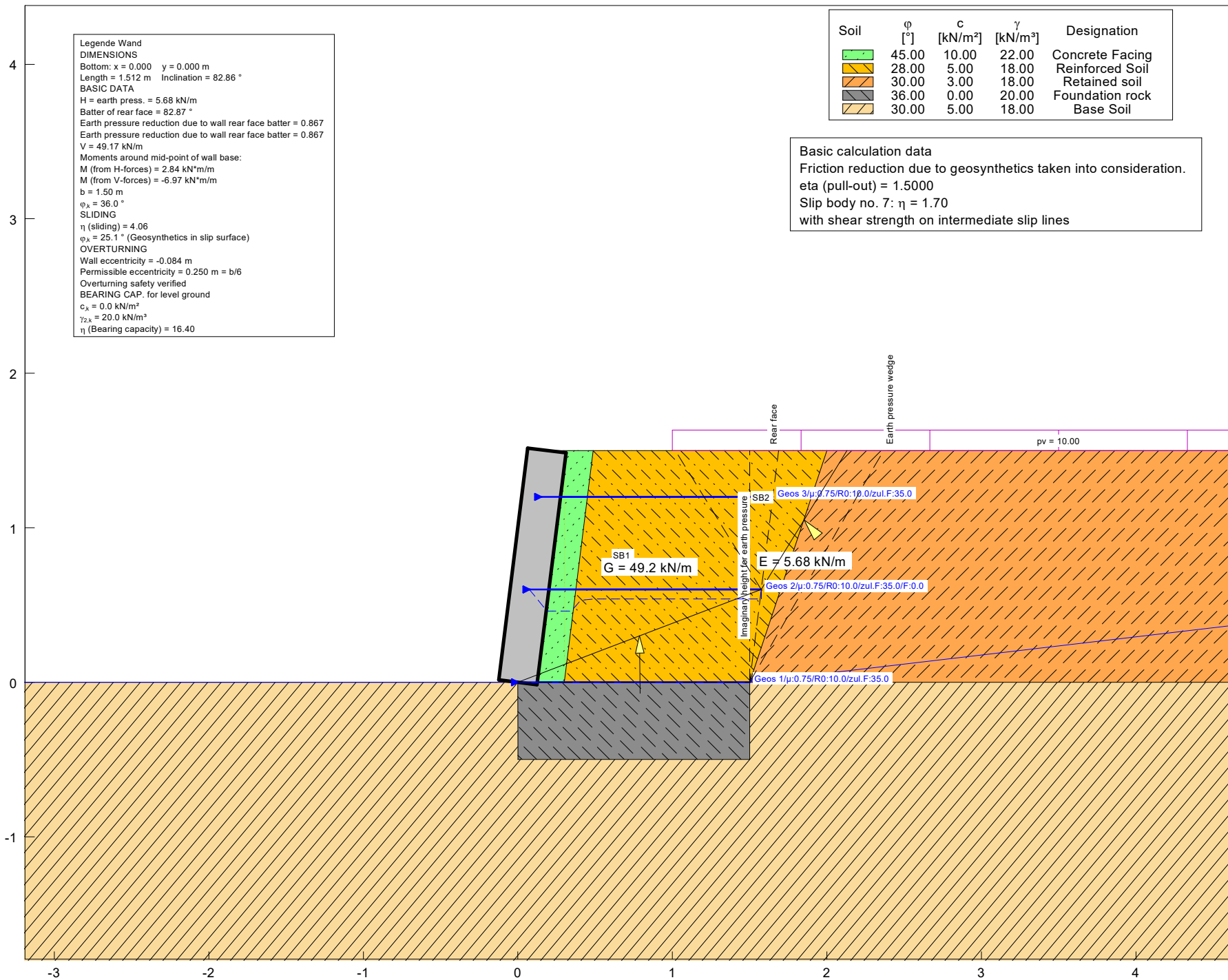
Appendix A1
Slope Stability Analysis
For
1.5m High Keystone Wall

Project:
Keystone wall at Sawah Lebar, K.Pilah, NS.

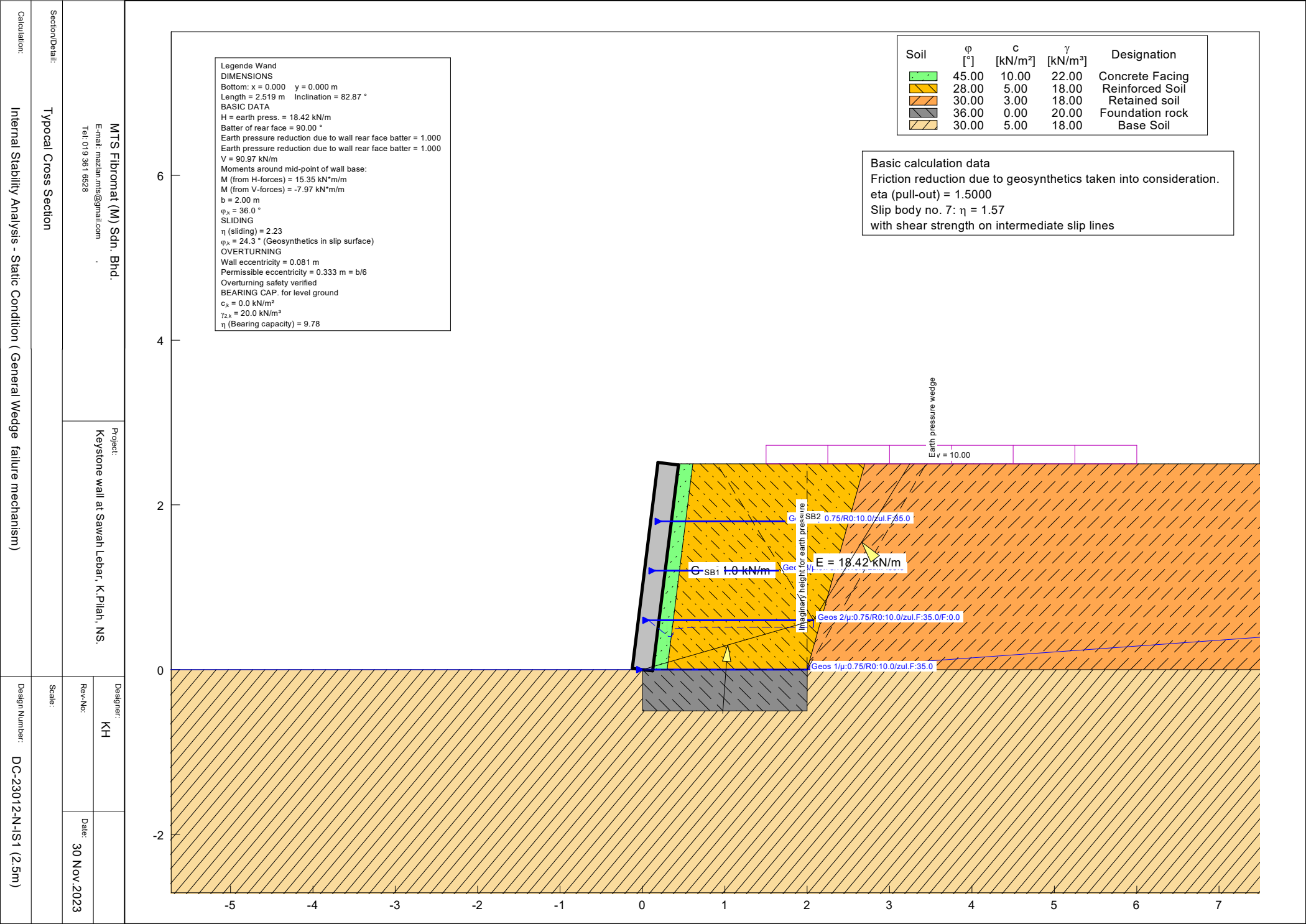
MTS Fibromat (M) Sc
E-mail: mazlan.mts@gmail.com
Tel: 019 361 6528

Design Number: DC-23012-N-IS1 (1.5m)

Typocal Cross Section



Appendix A2
Slope Stability Analysis
For
2.5m High Keystone Wall



Appendix A3
Slope Stability Analysis
For
3.5m High Keystone Wall

Typocal Cross Section

Scale:

Internal Stability Analysis - Static Condition (General Wedge failure mechanism)

Design Number: DC-23012-N-IS1 (3.5m)

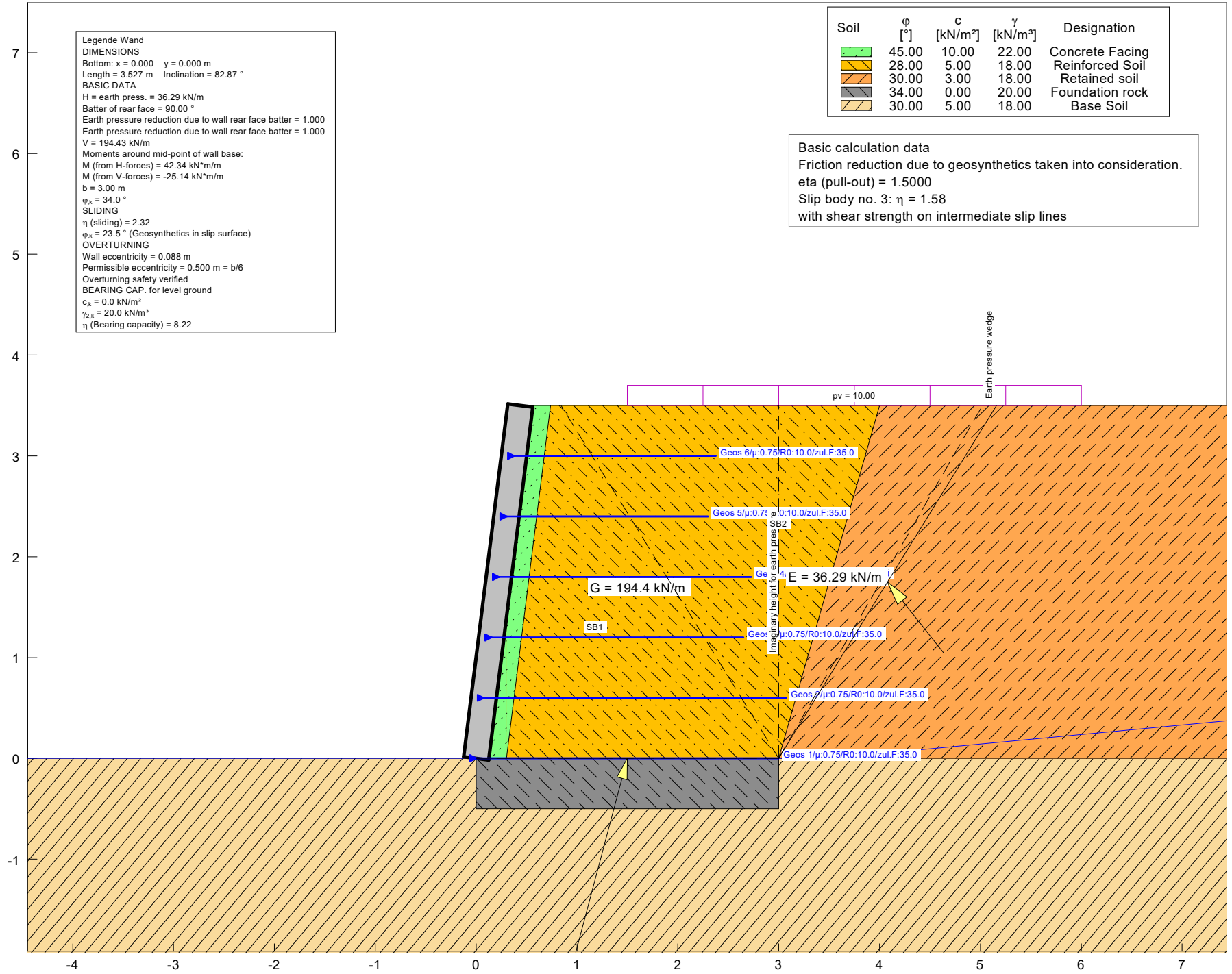
MTS Fibromat (M) Sdn. Bhd.
E-mail: mazlan.mts@gmail.com
Tel: 019 361 6528

Project:
Keystone wall at Sawah Lebar, K.Pilah, NS.

Designer: KH

Rev-No:

Date: 06/11/2000



Appendix B

Drawings

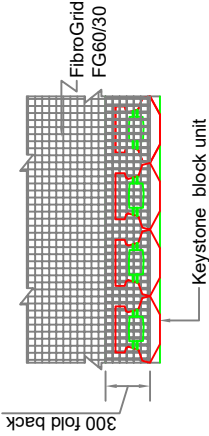
NOTES:

- 1) All dimension units are in millimetres unless otherwise specified.
- 2) This drawing shows some typical conceptual design details based on several assumptions, which shall be checked and verified and modified accordingly by qualified Professional Engineer before adoption for construction.
- 3) Detail construction drawing shall be prepared, checked and endorsed by qualified Professional Engineer after detail analysis based on adequate SI and site conditions, etc.
- 4) The internal stability for the proposed geogrid reinforced **wall/slope** is based on the following parameters:-
 - a) Approved suitable fill shall be used as reinforced soil. Minimum compaction shall be 95% B.S standard compaction. At least 1 field density shall be carried out per 250 cu.m of fill or as directed by the qualified site engineer.
 - b) Assumed design parameters:

Properties of soil			
	unit weight (kN/m³)	C' (kN/m²)	Phi (°)
Reinforced soil	18	5	28
Retained soil	18	3	30
Base soil	18	5	30

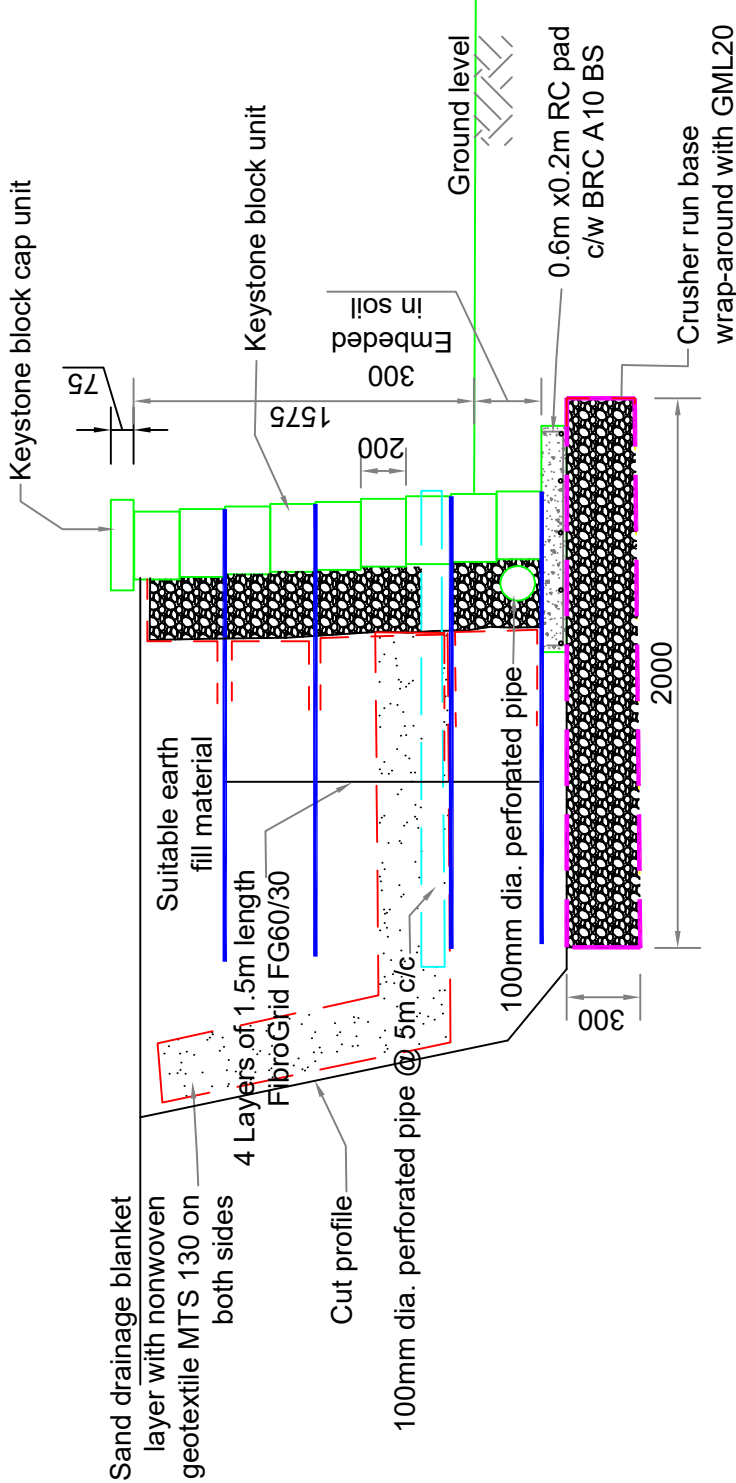
- c) CIU tests based on typical selected compacted samples shall be carried out to verify the assumptions (minimum 1 series of CIU tests per 500 cu.m of fill or as directed by the Engineer.
- 5) External stability for the geogrid reinforced **wall/slope** shall be checked and verified by the earthwork consultant after all survey data and confirmatory SI at site are completed and available.
- 6) All unsuitable material under the foundation for the proposed Fibromat reinforced geogrid-Fibrogid retaining wall system with Mackintosh Probe counts < 40 blow/ft shall be removed and replaced by crusher run material or as instructed by the qualified site engineer. Confirmatory Mackintosh probes and other suitable tests shall be carried out on site and decided by the qualified site engineer after site inspection.
- 7) Good temporary site drainage shall always be maintained to prevent any localized ponding. Fill material (before and after compaction) if exposed to rain and with moisture content exceeding optimum moisture + 3% shall be rejected or removed from the site immediately.
- 8) Locations for the proposed geogrid **reinforced wall/slope** shall be set-up at site by the contractor and verified by the qualified site engineer before carrying out the proposed confirmatory tests mentioned above.

Std Keystone Block size: 450x300x200
Capping Keystone size: 450x250x75



Details of Keystone Block Unit

Not to scale



Typical Section of Keystone Wall, 1.5m High.

Not to scale

Project:
PROPOSED KEYSTONE WALL FOR THE VERTICAL WALL NEAR THE DEWAN AT JALAN KG. GENTAM SEBERANG, SAWAH LEBAR, DAERAH KUALA PILAH, NEGERI SEMBILAN. (1.5m high exposed wall).

Title:
Keystone Reinforced Wall Details

Drawing No.:	Revision:	Date:
DC23012-N-D1	00	8-Dec-2023
MTS FIBROMAT (M) SDN BHD		
E-mail: mztan.mts@gmail.com		
Website: www.fibromat.com.my		
Tel: 019 3616528		

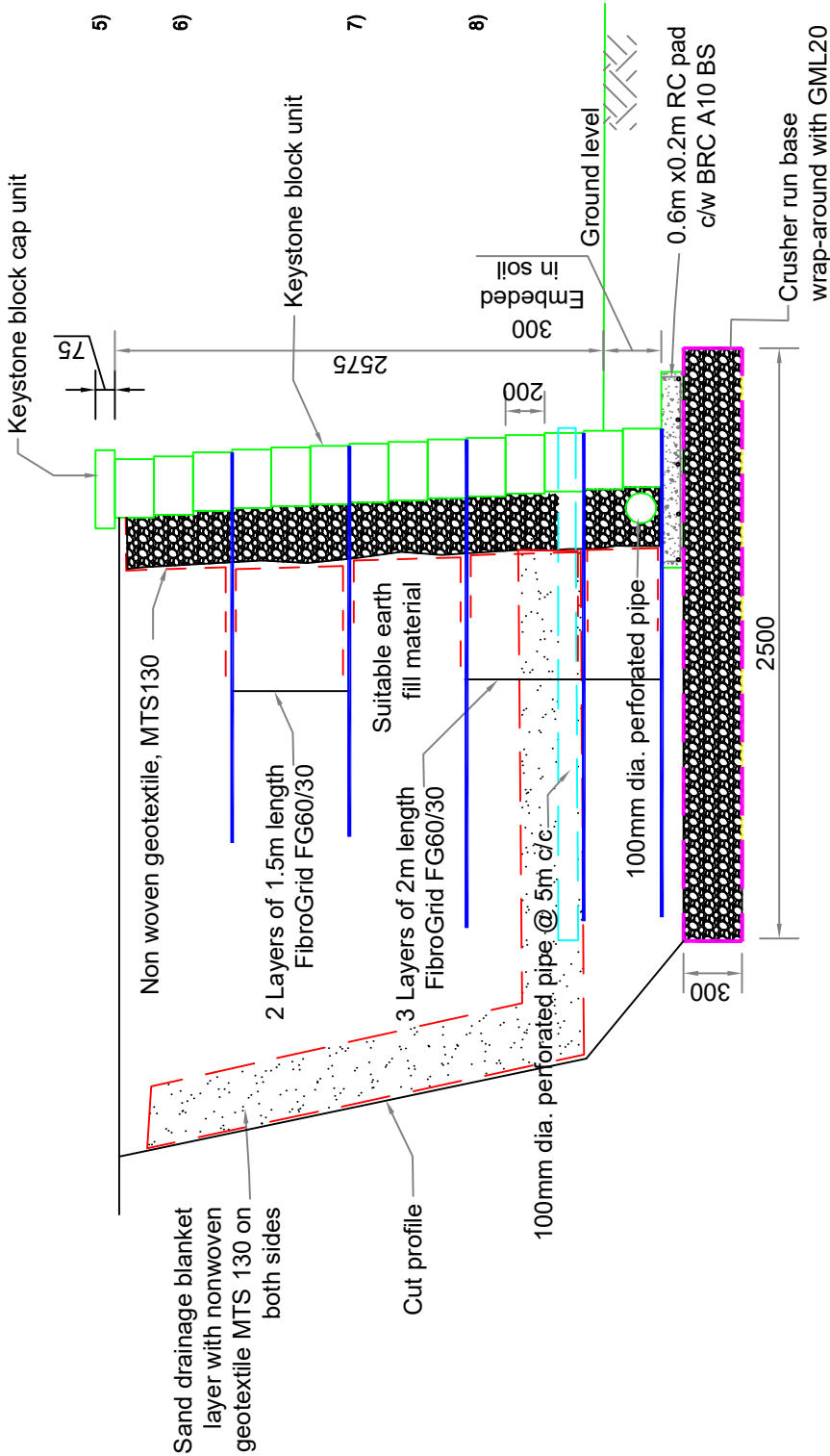
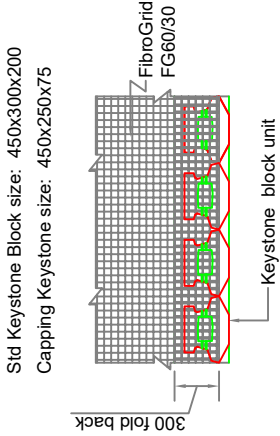
NOTES:

- 1) All dimension units are in millimetres unless otherwise specified.
- 2) This drawing shows some typical conceptual design details based on several assumptions, which shall be checked and verified and modified accordingly by qualified Professional Engineer before adoption for construction.
- 3) Detail construction drawing shall be prepared, checked and endorsed by qualified Professional Engineer after detail analysis based on adequate SI and site conditions, etc.
- 4) The internal stability for the proposed geogrid reinforced **wall/slope** is based on the following parameters:-
 - a) Approved suitable fill shall be used as reinforced soil. Minimum compaction shall be 95% B.S standard compaction. At least 1 field density shall be carried out per 250 cu.m of fill or as directed by the qualified site engineer.
 - b) Assumed design parameters:

Properties of soil			
	unit weight (kN/m³)	C' (kN/m²)	Phi (°)
Reinforced soil	18		28
Retained soil	18		30
Base soil	18		30
 - c) CIU tests based on typical selected compacted samples shall be carried out to verify the assumptions (minimum 1 series of CIU tests per 500 cu.m of fill or as directed by the Engineer.
 - d) External stability for the geogrid reinforced **wall/slope** shall be checked and verified by the earthwork consultant after all survey data and confirmatory SI at site are completed and available.
 - e) All unsuitable material under the foundation for the proposed Fibromat reinforced geogrid-Fibrogid retaining wall system with Mackintosh Probe counts < 40 blow/ft shall be removed and replaced by crusher run material or as instructed by the qualified site engineer. Confirmatory Mackintosh probes and other suitable tests shall be carried out on site and decided by the qualified site engineer after site inspection.
 - f) Good temporary site drainage shall always be maintained to prevent any localized ponding. Fill material (before and after compaction) if exposed to rain and with moisture content exceeding optimum moisture + 3% shall be rejected or removed from the site immediately.
 - g) Locations for the proposed geogrid **reinforced wall/slope** shall be set-up at site by the contractor and verified by the qualified site engineer before carrying out the proposed confirmatory tests mentioned above.

Details of Keystone Block Unit

Not to scale



Typical Section of Keystone Wall, 2.5m High.

Not to scale

Project:
PROPOSED KEYSTONE WALL FOR THE VERTICAL WALL NEAR THE DEWAN AT JALAN KG. GENTAM SEBERANG, SAWAH LEBAR, DAERAH KUALA PILAH, NEGERI SEMBILAN. (2.5m high exposed wall).

Title:
Keystone Reinforced Wall Details

Drawing No.:	Revision:	Date:
DC23012-N-D2	00	8-Dec-2023
MTS FIBROMAT (M) SDN BHD		
E-mail: mztlan.mts@gmail.com		
Website: www.fibromat.com.my		
Tel: 019 3616528		

NOTES:

- 1) All dimension units are in millimetres unless otherwise specified.
- 2) This drawing shows some typical conceptual design details based on several assumptions, which shall be checked and verified and modified accordingly by qualified Professional Engineer before adoption for construction.
- 3) Detail construction drawing shall be prepared, checked and endorsed by qualified Professional Engineer after detail analysis based on adequate SI and site conditions, etc.
- 4) The internal stability for the proposed geogrid reinforced **wall/slope** is based on the following parameters:-
 - a) Approved suitable fill shall be used as reinforced soil. Minimum compaction shall be 95% B.S standard compaction. At least 1 field density shall be carried out per 250 cu.m of fill or as directed by the qualified site engineer.
 - b) Assumed design parameters:

Properties of soil			
	unit weight (kN/m³)	C' (kN/m²)	Phi (°)
Reinforced soil	18	5	28
Retained soil	18	3	30
Base soil	18	5	30
 - c) CIU tests based on typical selected compacted samples shall be carried out to verify the assumptions (minimum 1 series of CIU tests per 500 cu.m of fill or as directed by the Engineer.
 - 5) External stability for the geogrid reinforced **wall/slope** shall be checked and verified by the earthwork consultant after all survey data and confirmatory SI at site are completed and available.
 - 6) All unsuitable material under the foundation for the proposed Fibromat reinforced geogrid-Fibrogrid retaining wall system with Mackintosh Probe counts < 60 blow/ft shall be removed and replaced by crusher run material or as instructed by the qualified site engineer. Confirmatory Mackintosh probes and other suitable tests shall be carried out on site and decided by the qualified site engineer after site inspection.
 - 7) Good temporary site drainage shall always be maintained to prevent any localized ponding. Fill material (before and after compaction) if exposed to rain and with moisture content exceeding optimum moisture + 3% shall be rejected or removed from the site immediately.
 - 8) Locations for the proposed geogrid **reinforced wall/slope** shall be set-up at site by the contractor and verified by the qualified site engineer before carrying out the proposed confirmatory tests mentioned above.

Properties of soil

	unit weight (kN/m³)	C' (kN/m²)	Phi (°)
Reinforced soil	18	5	28
Retained soil	18	3	30
Base soil	18	5	30

- 1) All dimension units are in millimetres unless otherwise specified.
- 2) This drawing shows some typical conceptual design details based on several assumptions, which shall be checked and verified and modified accordingly by qualified Professional Engineer before adoption for construction.
- 3) Detail construction drawing shall be prepared, checked and endorsed by qualified Professional Engineer after detail analysis based on adequate SI and site conditions, etc.
- 4) The internal stability for the proposed geogrid reinforced **wall/slope** is based on the following parameters:-
 - a) Approved suitable fill shall be used as reinforced soil. Minimum compaction shall be 95% B.S standard compaction. At least 1 field density shall be carried out per 250 cu.m of fill or as directed by the qualified site engineer.
 - b) Assumed design parameters:

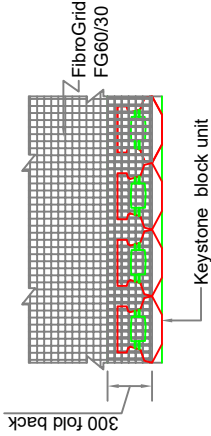
Properties of soil			
	unit weight (kN/m³)	C' (kN/m²)	Phi (°)
Reinforced soil	18	5	28
Retained soil	18	3	30
Base soil	18	5	30
 - c) CIU tests based on typical selected compacted samples shall be carried out to verify the assumptions (minimum 1 series of CIU tests per 500 cu.m of fill or as directed by the Engineer.
 - 5) External stability for the geogrid reinforced **wall/slope** shall be checked and verified by the earthwork consultant after all survey data and confirmatory SI at site are completed and available.
 - 6) All unsuitable material under the foundation for the proposed Fibromat reinforced geogrid-Fibrogrid retaining wall system with Mackintosh Probe counts < 60 blow/ft shall be removed and replaced by crusher run material or as instructed by the qualified site engineer. Confirmatory Mackintosh probes and other suitable tests shall be carried out on site and decided by the qualified site engineer after site inspection.
 - 7) Good temporary site drainage shall always be maintained to prevent any localized ponding. Fill material (before and after compaction) if exposed to rain and with moisture content exceeding optimum moisture + 3% shall be rejected or removed from the site immediately.
 - 8) Locations for the proposed geogrid **reinforced wall/slope** shall be set-up at site by the contractor and verified by the qualified site engineer before carrying out the proposed confirmatory tests mentioned above.

Project:
PROPOSED KEYSTONE WALL FOR THE VERTICAL WALL NEAR THE DEWAN AT JALAN KG. GENTAM SEBERANG, SAWAH LEBAR, DAERAH KUALA PILAH, NEGERI SEMBILAN. (3.5m high exposed wall).

Title:
Keystone Reinforced Wall Details

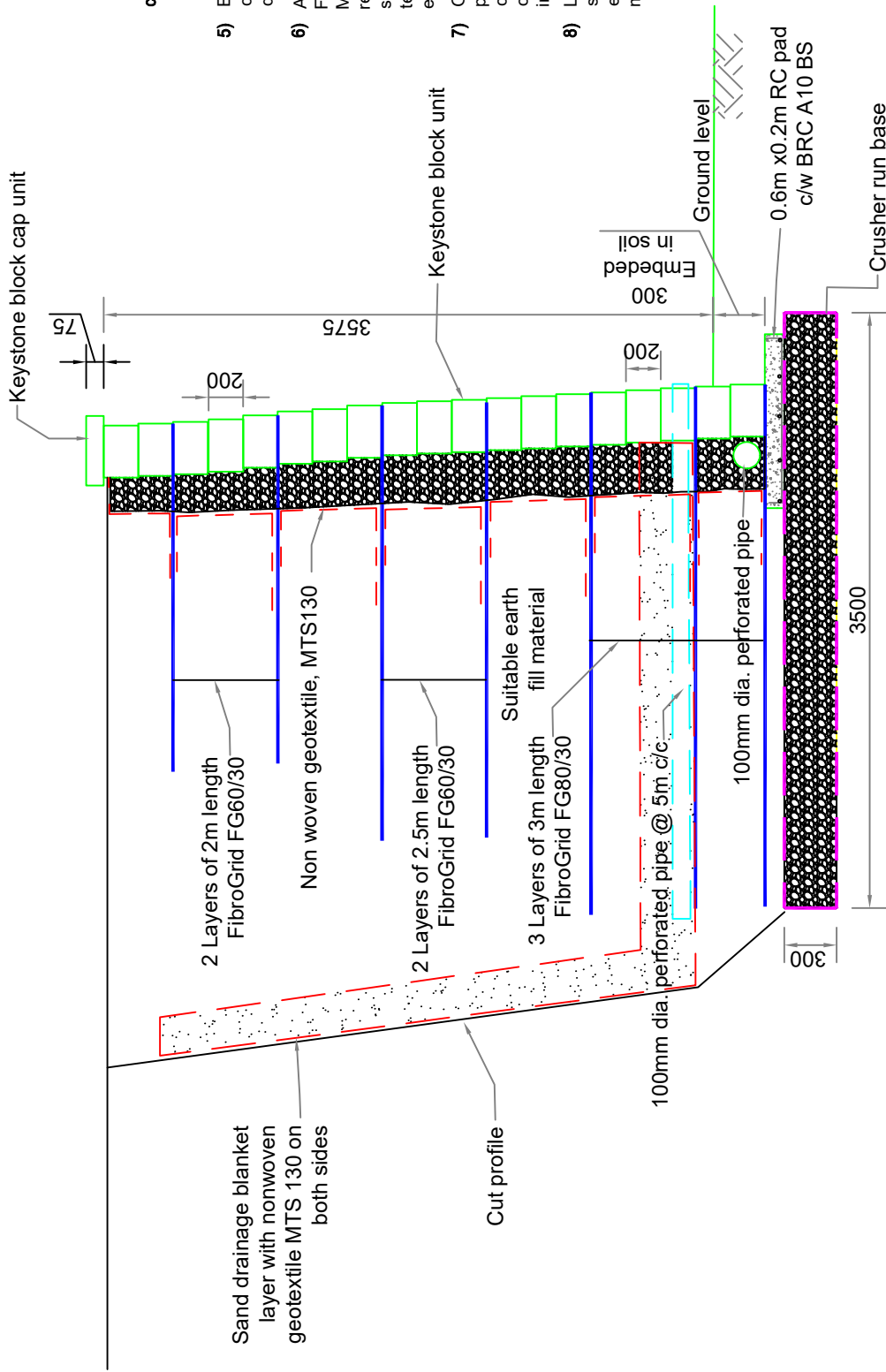
Drawing No.:	Revision:	Date:
DC23012-N-D3	00	8-Dec-2023
MTS FIBROMAT (M) SDN BHD		
E-mail: mztlan.mts@gmail.com		
Website: www.fibromat.com.my		
Tel: 019 3616528		

Std Keystone Block size: 450x300x200
Capping Keystone size: 450x250x75



Details of Keystone Block Unit

Not to scale



Typical Section of Keystone Wall, 3.5m High.

Not to scale

Appendix C

Specification



integrated brickworks sdn. bhd.



"Where Excellence is the Standard"

In a market filled with choices, KEYSTONE® still sets the standard.

In today's world of imitation products, KEYSTONE® Retaining Wall Systems continues to set the worldwide benchmark for segmental retaining wall performance and aesthetics, from soil retention and erosion control to landscape systems in over 35 countries.

As the industry's leader, KEYSTONE® has set the bar for excellence, innovation and technology within the industry, being a product of the energy, passion and focus of the people dedicated to ensure that KEYSTONE® products and services offer the best site solutions for residential, commercial, recreational, industrial and institutional applications.

KEYSTONE® created the mortar-less segmental retaining wall market with its patented interlocking modular design nearly two decades ago, and still retains that distinct difference of outer beauty and inner strength - the beauty of natural stone, the durability of granite and positive unit connection internally.



The high-strength KEYSTONE® concrete units and patented interlocking pin system ensures a conveniently easy positive connection between KEYSTONE® units, as well as between the KEYSTONE® units and soil reinforcement elements. Additionally, KEYSTONE® patented pins aid in horizontal alignment, while also providing the ability to vary the wall set-back, a feature unique to KEYSTONE®.

PRODUCT ATTRIBUTES



KEYSTONE® Retaining Wall modular units are dry-cast from high strength low absorption concrete for durable long-lasting structures.

All structural units are interconnected using high-strength fiberglass pins for strong shear connection. The connecting pins allow for ease of unit alignment and a secure positive mechanical connection with soil reinforcement.

KEYSTONE®'s patented pin system is the critical difference providing the safety and security of a structurally sound retaining wall solution.

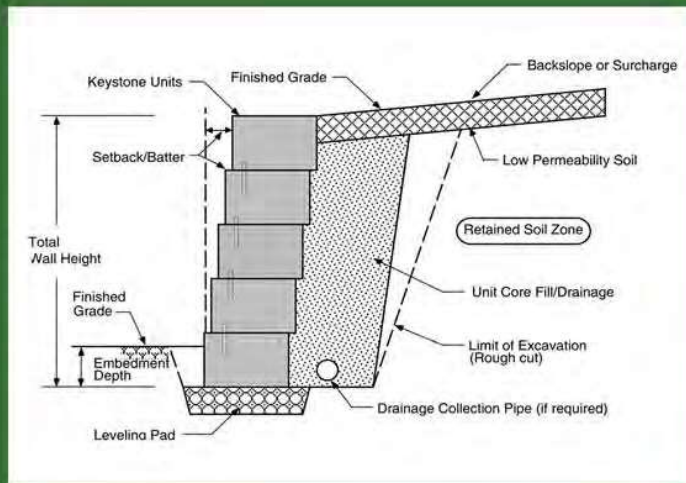
KEY FEATURES

- High Strength Pin Connection System
- Open Cores
 - Increases vertical drainage through the face units
 - Allows for block interlock across block interfaces
 - Improves connection strength between blocks and geogrid
 - Improves unit-to-unit shear resistance
- Versatility
 - Multiple fascia and texture options
 - "Near" vertical or battered set-back options are built-in
- Shape
 - Tapered sides allow easy curves, corners and circles
 - Units have a "tail" element for easy handling



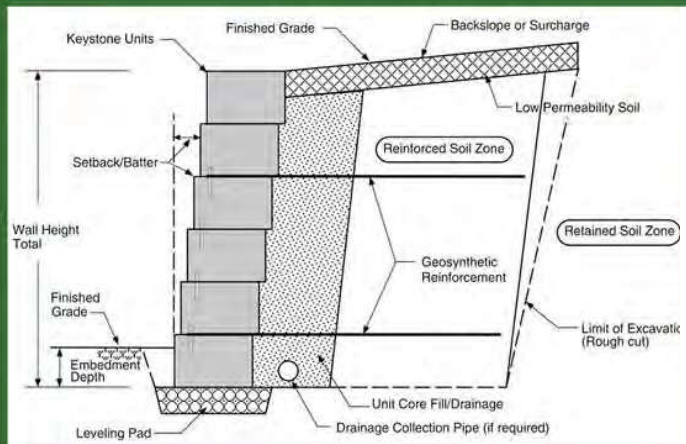
THE “TECHNICAL” SIDE

The two most common retaining wall structures built today are simple gravity walls and larger reinforced soil wall structures. KEYSTONE® offers products and specific design methodology for both structure types.



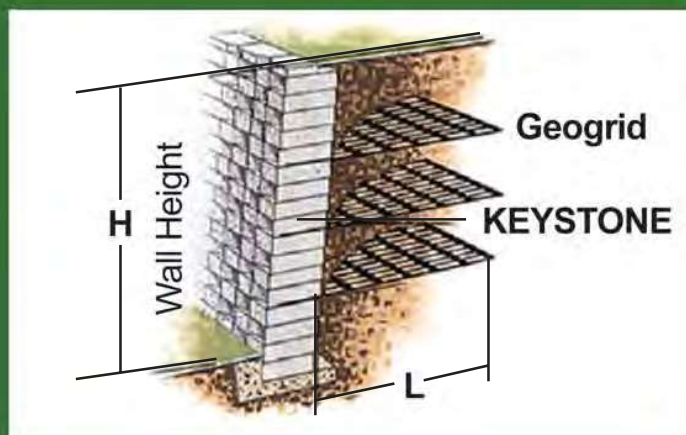
GRAVITY WALLS

One of the most basic types of retaining walls, the gravity wall, relies on its mass and cross-sectional geometry to resist the earth pressure that is exerting force to move the structure in a lateral direction. KEYSTONE® retaining walls are able to resist lateral pressure with their unit weight and deep embedment shape. The mortarless, yet structurally interconnected KEYSTONE® units also promote the freeflow of water drainage to prevent hydrostatic loads. For low, non-critical applications, KEYSTONE® products make highly cost-effective gravity wall structures.



REINFORCED SOIL WALLS

For taller or more critical walls, KEYSTONE® units are combined with soil reinforcement options (such as extensible geogrids, geotextiles, inextensible tie-back anchor or galvanized steel ladder reinforcing) to create large composite structures. With this properly designed combination, the reinforced soil mass can support greater earth pressures and surcharge loads. These composite structures have permitted the construction of retaining wall structures over 18m high.



GEOGRID REINFORCEMENT

Since the early 1980's, geogrids have proven successful in providing durable soil reinforcement for the retaining wall industry. Proper design methodology today incorporates the laboratory tested connection strength of concrete units and geogrid in all reinforced retaining wall systems. Geogrids are made from high-density polyolefins or high-tenacity woven polyester yarns.

THE KEYSTONE® RANGE

RETAINING WALL SYSTEMS
A CONTECH COMPANY

Retaining Excellence™



STANDARD UNIT

Size (HxWxD)	8" x 18" x 21½" (±0.203 x 0.457 x 0.546m)
Exposed Face Area	1 ft²/pc : 8" x 18" (0.093 m²/pc)
Unit Weight	±42.8 kg. *



COMPAC UNIT

Size (HxWxD)	8" x 18" x 12" (±0.203 x 0.457 x 0.305m)
Exposed Face Area	1 ft²/pc : 8" x 18" (0.093 m²/pc)
Unit Weight	±38.5 kg *



MINI & CAP UNIT

Size (HxWxD)	4" x 18" x 10½" (±0.102 x 0.457 x 0.267m)
Exposed Face Area	½ ft²/pc : 4" x 18" (0.046 m²/pc)
Unit Weight	±20.2 kg. *

* depending on variation of unit type, fascia treatment and soil reinforcement elements



CONNECTION PIN

Size (HxWxD)	½" Ø x 5¼" (±0.012 Ø x 0.133m)
Flexural Strength	Min 128,000 psi (±883 MPa)
Short Beam Shear Strength	Min 6,400 psi (±44 MPa)

Product reference standards : MS 27 : 1996, BS 6073 : 1981, ASTM C 1372-97, ASTM C 90-75, ASTM C 140-75

Additional information, products and services available on request

- | | |
|---|--|
| a.... Technical Brochure | g.... Specialist Tools and Handling Equipment |
| b.... KeyWall Design Software | h.... Method Statement |
| c.... KeyWall Design Manual and Operating Guide | i.... Preliminary Design Assistance |
| d.... KEYSTONE® Specifications | j.... Installation Guidelines |
| e.... Geogrid Specifications | k.... Contractors' Installation Training Programme |
| f.... Segmental Retaining Wall (SRW) Specifications | l.... Construction Detail |



**Sg Kerayong
Kuala Lumur**



**Blue Lagoon
Port Dickson**



**Tmn Tasik Perdana
Kuala Lumpur**



Bukit Jelutong



**Kinrara Golf Club
Bdr Kinrara**



**Seksyen 7
Shah Alam**



**Mahkamah Tinggi
Shah Alam**



**Masjid Wilayah
Kuala Lumpur**



**Guthrie Corridor
Expressway**



**Butterworth
Outer Ring Road**



**Jalan Tun Razak
Kuala Lumpur**



**Upland Gyrratory
Kuching**

integrated brickworks sdn bhd

(012282-X)

No 27, Jalan Tago 5, Tago Industrial Park
Kepong, Bdr Sri Damansara, 52200 Kuala Lumpur, Malaysia.

Tel: 603 - 6272 4631 Fax: 603 - 6272 4681

www.integratedbrickworks.com

Licensee for Malaysia, Singapore and Brunei Darussalam